

# IT7 (SSI) Progress Report

## Seismic Pressures on Retaining Walls

Stephen Harris (Chair), CB Crouse, Gyimah Kasali,  
Bruce Kutter, Armin Masroor, Ian McFarlane, Bob  
Pekelnicky, Jon Stewart

# August 2017 PUC Meeting

## **PUC Agreed with IT7 recommendation to:**

Strike from Resource Paper 12 the section on seismic earth pressure

Prepare a new resource paper on the topic of seismic earth pressures.

# Nov 2017 PUC Meeting

## **Proposed approach outlined in brief:**

External elements attached to wall – requires SSI analysis, existing procedures sufficient

Free-standing walls: kinematic approach, general framework described (depends on frequency content of ground motion and wall height)

# This Meeting

Analysis procedure for free-standing walls

Effect of wall inertia (free-standing walls)

Feedback request

Path forward

# This Meeting

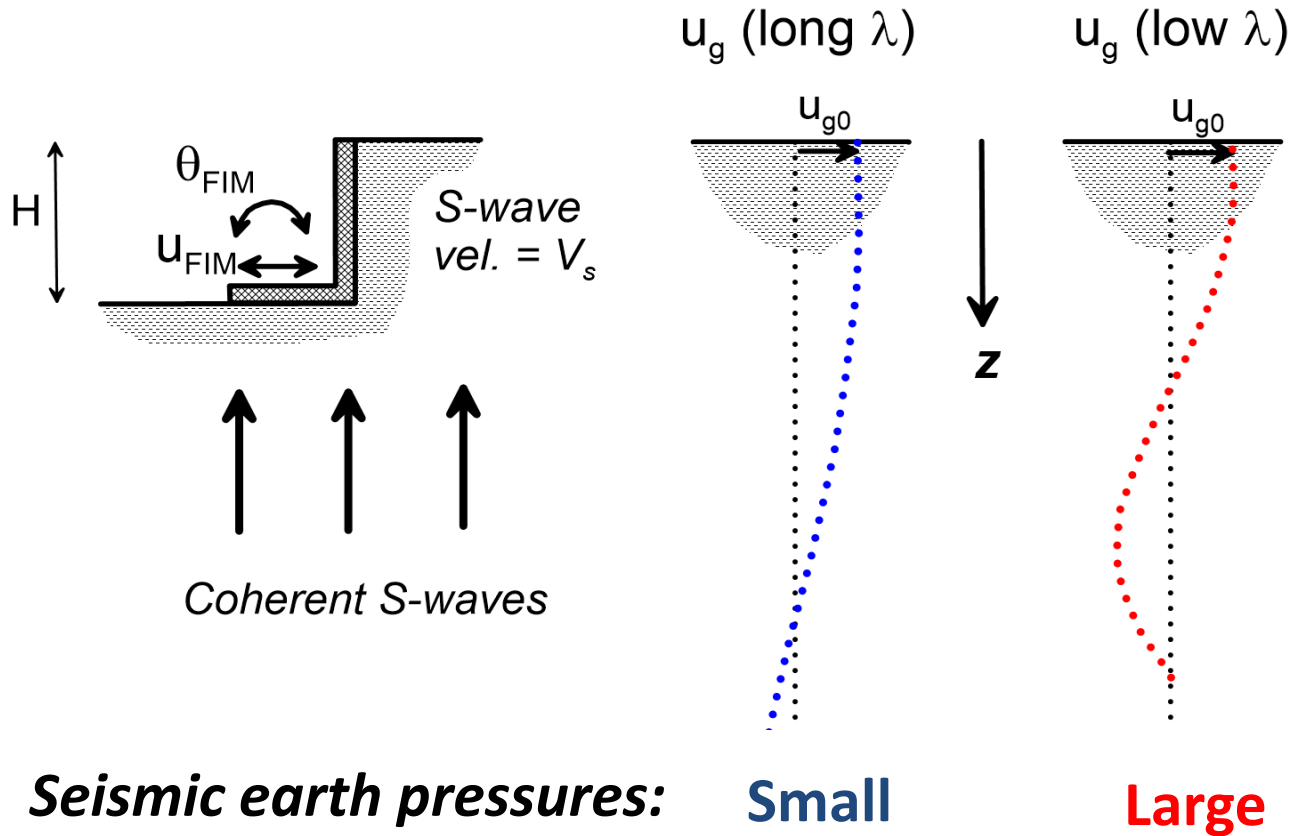
## **Analysis procedure for free-standing walls**

Effect of wall inertia (free-standing walls)

Feedback request

Path forward

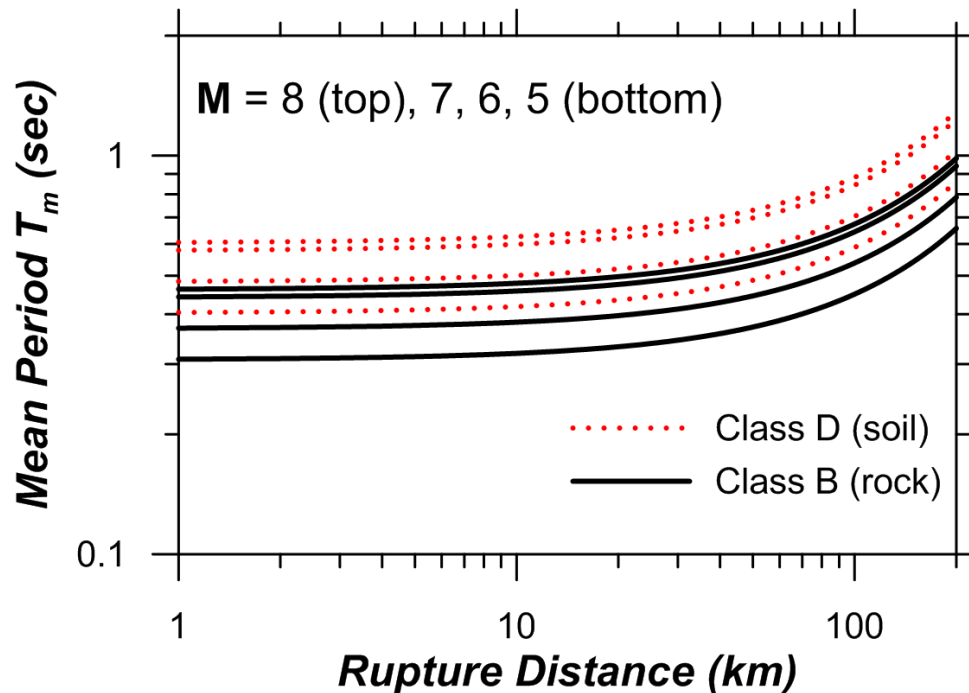
# Basis



$$\lambda = V_{s,av} T_m$$

Reasonable range of  $V_{s,av}$ : 100 – 300 m/s

Reasonable range of  $T_m$ : 0.3-0.7 sec (WUS)



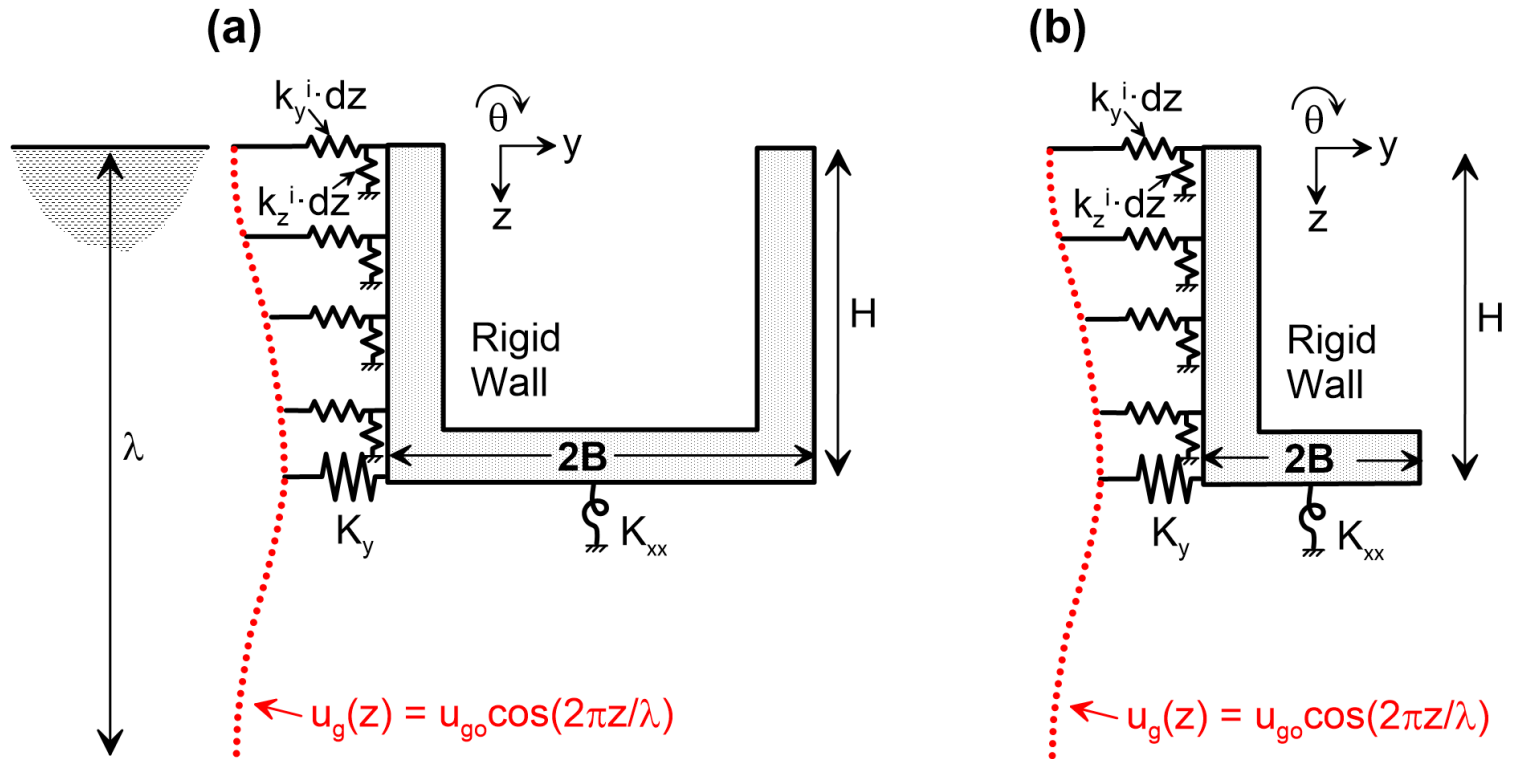
Rathje et al. 2004 GMM

## Approximate Range of Wavelengths, $\lambda$

$V_{S,av} \setminus T_m$	0.3 sec	0.5 sec	0.7 sec
100 m/s	30 m	50 m	70 m
200 m/s	60 m	100 m	140 m
300 m/s	90 m	150 m	210 m



# System Considered



Foundation dimensions usually taken as half-widths,  $B$ , in impedance function solutions

# Logic Behind Procedure

Ground motion represented by a single frequency and suitable ground motion amplitude (related to PGV)

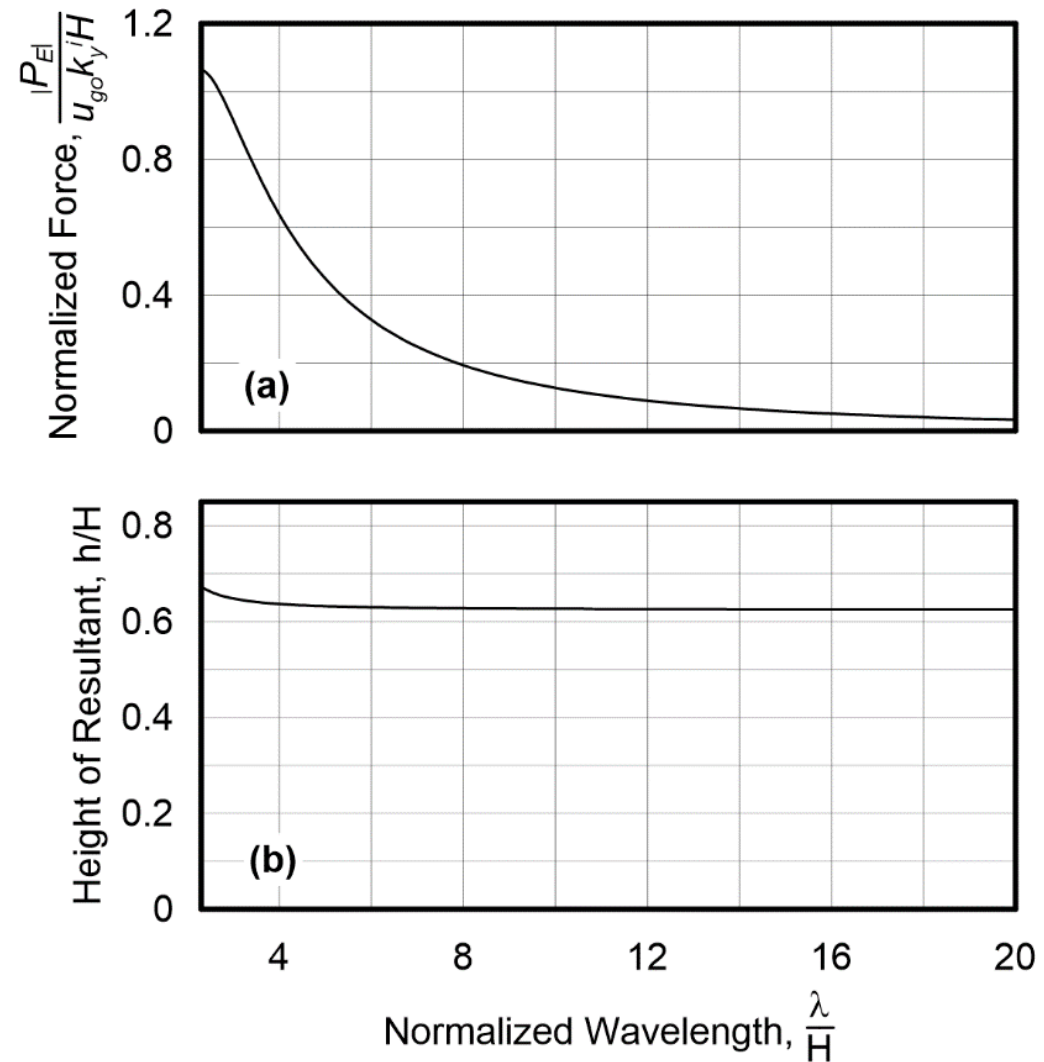
Backfill  $V_s$  profile & analysis of how reduced by modulus reduction effects

Evaluate wall-soil interface stiffness,  $k_y^i$

Normalized seismic wall force evaluated as function of  $\lambda/H = V_{s,av}T_m/H$ . Optional factors can be considered to reduce normalized force

De-normalization using amplitude,  $k_y^i, H$

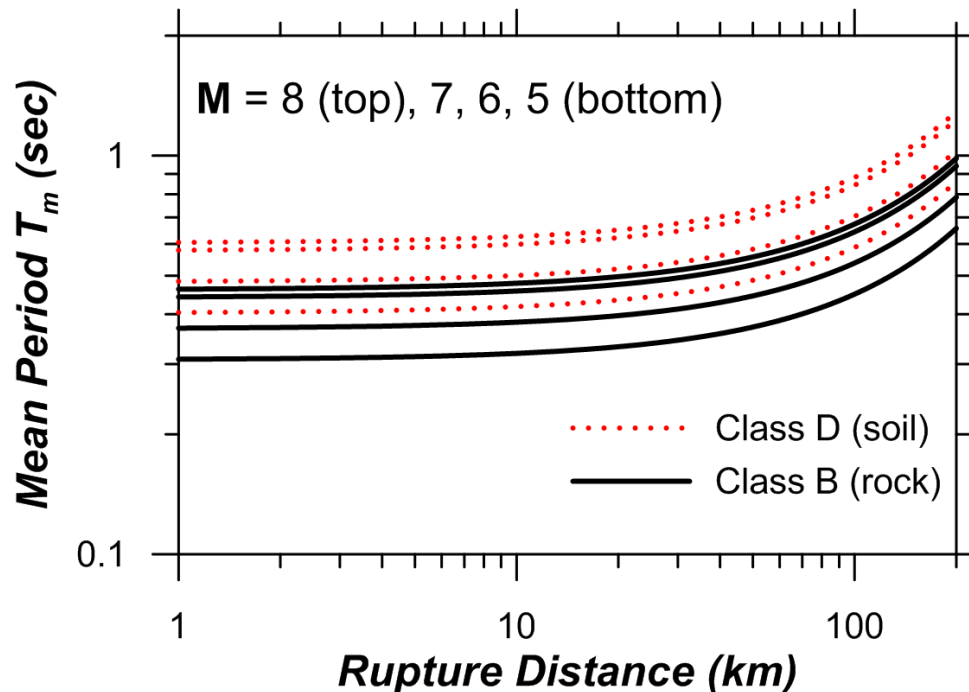
- Fixed base
- Rigid wall
- Uniform backfill



$$\frac{|P_E|}{k_y^i u_{g0}^i H} = \cos(kH) - \frac{\sin(kH)}{kH}$$

# Detailed Procedure

1. Seismic hazard analysis to obtain PGV
2. Identify **M** and R related to PGV (de-aggregation, or deterministic values). Estimate  $T_m$ . Compute corresponding angular frequency  $\omega_m = 2\pi/T_m$ .



Rathje et al. 2004 GMM

# Detailed Procedure

3. Measure or estimate  $V_s$  profile. Adjust for strain effects. Average of reduced profile is  $V_{s,av,r}$

4. Potentially neglect seismic earth pressures if  $\lambda/H = V_{s,av,r}T_m/H$  exceeds some value (perhaps 15)

e.g., 200 m/s,  $T_m = 0.5$  sec,  $H = 5$ m

$$\lambda/H = 20$$

5. Estimate ground motion amplitude,  $u_{g0} = f_u PGV / \omega_m$ , where factor  $f_u$  depends on  $\lambda/H$  (generally between 0.65 and 0.95).

# Detailed Procedure

6. Compute normalized force amplitude,  $|P_E|/(k_y^i u_{g0} H)$ . Adjust for flexible base, inhomogeneous backfill, flexible wall, if desired (reduces force)

7. Compute  $k_y^i$ , depends on

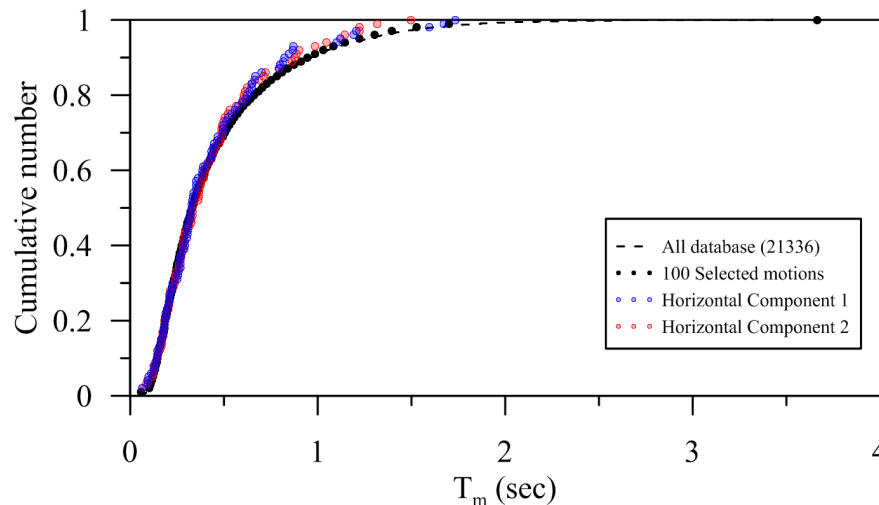
$$k_y^i = \frac{\pi}{\sqrt{(1-\nu)(2-\nu)}} \frac{G}{H} \sqrt{1 - \left(\frac{4H}{\lambda}\right)^2}$$

where  $G = \rho V_{S,av,r}^2$  and  $\nu$  = soil Poisson's ratio

8. De-normalize  $P_E$ .

# Verification

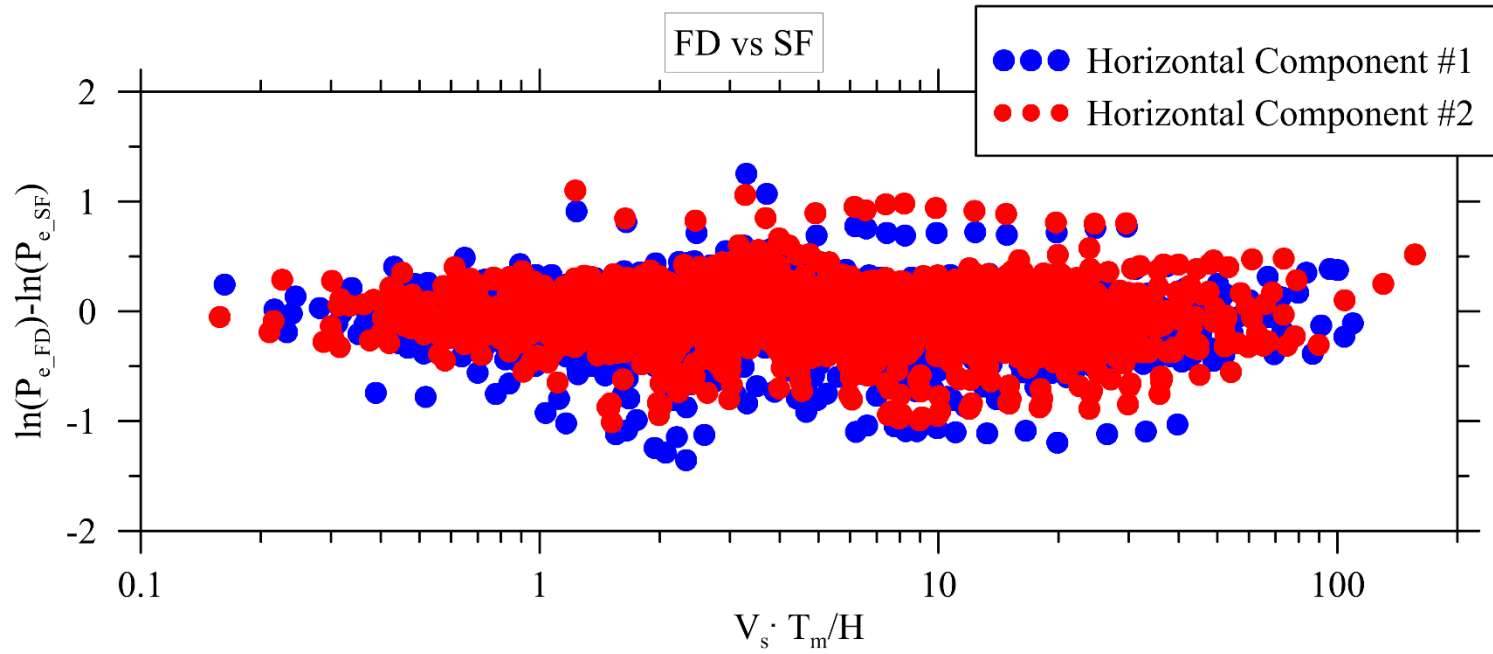
Full Fourier analysis vs use of single frequency  
ground motions selected to have range of  $T_m$ :



Each motion run through wall – full analysis

Single frequency method used with motions  
amplitude and mean period

Difference computed:  $\ln(P_{E,full}) - \ln(P_{E,sf})$

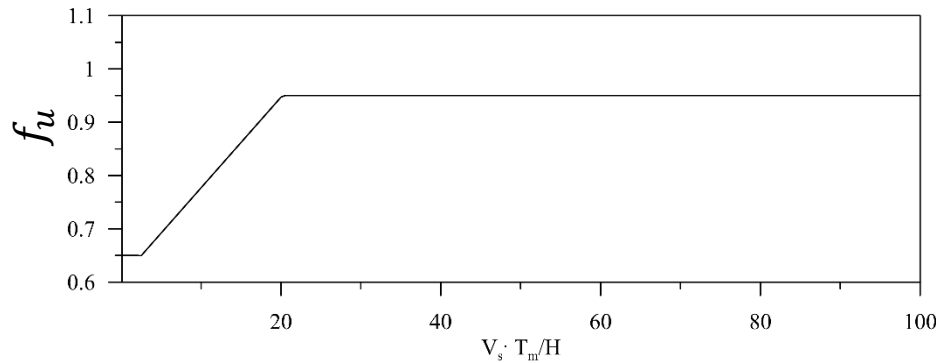


Standard deviation Horizontal Component #1: 0.2808

Standard deviation Horizontal Component #2: 0.2776

Mean error Horizontal Component #1: -0.0556

Mean error Horizontal Component #2: -0.0299



$$f_u = \begin{cases} 0.65 & \text{for } \frac{V_s T_m}{H} < 2.5 \\ 0.017 * \frac{V_s T_m}{H} + 0.607 & \text{for } 2.5 \leq \frac{V_s T_m}{H} \leq 20 \\ 0.95 & \text{for } \frac{V_s T_m}{H} > 20 \end{cases}$$



# This Meeting

Analysis procedure for free-standing walls

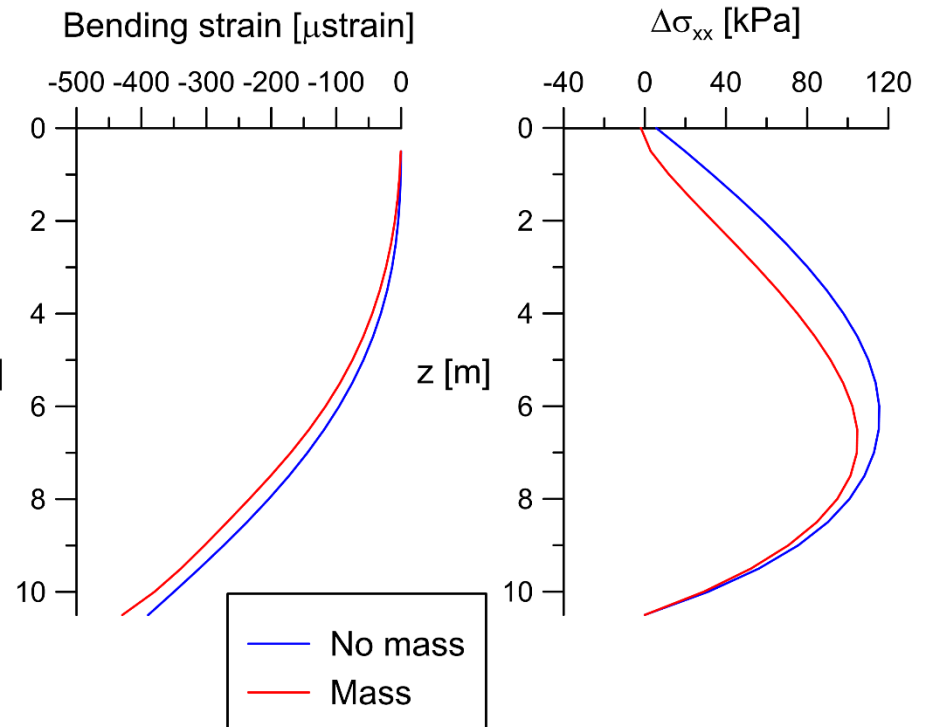
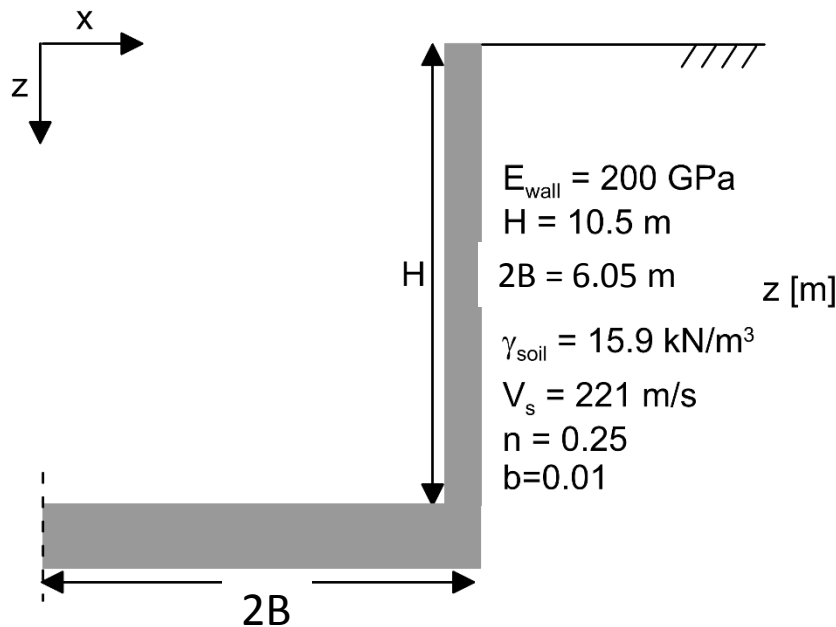
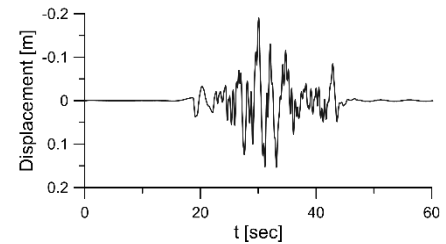
**Effect of wall inertia (free-standing walls)**

Feedback request

Path forward

# Effect of wall inertia

Ground motion:  
Loma Prieta Earthquake 1989  
(LGPC Station) – PGA 1.05g



Difference at the peak  $\simeq 9\%$

Configuration: flexible wall, rigid base, inhomogeneous soil deposit

# This Meeting

Analysis procedure for free-standing walls

Effect of wall inertia (free-standing walls)

**Feedback request**

Path forward

This procedure is a significant departure from current practice

Parameter	MO Approach	SSI Approach*
Wall dimensions	H	H, B
Soil stiffness	--	Vs,av
Soil strength	$\phi$ , c	--
Ground motion amplitude	PGA	PGV
Ground motion frequency	--	Tm

\* Simplest case (fixed base, rigid wall, uniform soil)

Feedback requested on whether this appears reasonable from point of view of:

- Developing input parameters
- Calculation effort

# Path Forward

- Example computations for various values of  $V_s$ , distance to source, and magnitude. Offset of decrease of amplitude and increase of period with distance.
- Compare some examples to M-O case.
- Develop model & description (Part 3 paper) for balloting in IT7

**End**